

## EFFECT THE SHAPE OF STILLING BASIN ON SCOUR DOWNSTREAM HEADING-UP STRUCTURES

تأثير شكل أحواض التهدئة على النحر خلف منشآت الحجز

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خلاصة :

يختص هذا البحث بدراسة مدى تأثير شكل أحواض التهدئة على خواص السربان و النحر خلف منشآت الحجز . تم تصميم ستة وعشرون نموذج مختلف الشكل لحوض التهدئة خلف الهبار . تم إجراء اثنين وسبعين تجربة على الاشكال المختلفة لأحواض التهدئة مع تغيير خواص السربان المختلفة . وأمكن بهذه الدراسة ربط العلاقات الناجمة عن خواص السربان و اشكال أحواض التهدئة بمقاييس بيانية و نأمل أن تكون هذه الدراسة مفيدة لمصممى أحواض التهدئة خلف منشآت الحجز .

### ABSTRACT

The objective of this research is to investigate how the shape of stilling basin affects the flow and the principal dimension of local scour downstream heading up Structures. Twenty-six type shapes of stilling basin have been designed in conjunction with weirs. Seventy-two runs were conducted considering various stilling basin shape with different flow conditions. Relationship between the shape of the stilling basin, flow conditions and scour parameters were developed from the data obtained from experimental tests.

### INTRODUCTION

Many laboratory studies have been made on stilling basin to investigate the efficiency of anti-scour designs downstream of hydraulic structures. To mention a few of the experimenters who contributed basic information there are Bradley and Peterka, U.S. Bureau of Reclamation, Laursen and others, Mohamed Ali, Peterka, Uymaz, Pillai et al, Breusers and Raudkivi, Baghdadi, Hoffmans. There is probably no phase of stilling basin that has received more attention, yet, from a practical viewpoint, there is still much to be worked. Therefore, it was the objective of this study to investigate how the shape of stilling basin affects the flow and the characteristic of scour downstream heading up structures. In this paper twenty sixth type shape of stilling basin were used to minimize the scour depth and to develop the best shape of stilling basin. The present experimental program include the experiments for investigate the changes of shape and dimension of stilling basin.

## DIMENSIONAL ANALYSIS

Experimental program require correlation between the gathering of data and its graphical representation. In the present study the independent variables considered in the dimensional analysis are chosen to represent the experimental condition of the problem,

Referring to Fig (1), the following parameters are used,

$q$ : discharge per unit width of flume

$D$ : the tail water depth.

$d_s$ : the maximum scour depth.

$F_r$ : Froude number =  $v/(g \cdot D)^{0.5}$ .

$L_s$ : the length of scour hole.

$L_m$ : the length to the location of maximum scour depth.

The variables are numerous and preclude rigorous mathematical analysis therefore; a model study was performed based on the principles of similitude to obtain and empirically analyze the data. The scour hole dimension of  $d_s$ ,  $L_s$ , and  $L_m$  were converted to dimensionless parameters  $d_s/D$ ,  $L_s/D$ ,  $L_m/D$  and the kinetic flow factor  $1/F_r^2$ .

## EXPERIMENTAL SET-UP

A series of experiments were conducted in a plexiglass flume 4.80 m long, 0.075m wide, and 0.17 m deep, of a circulating type. The flume was divided into an upstream section, a test reach, and a downstream section, as shown in Fig. (2). The upstream section consisted of a reception area for the laboratory water and a solid horizontal bed. To dampen disturbance and to provide an even flow distribution across the flume a perforated screen was placed in the upstream section before the upstream weir. The fixed bed was situated 0.05 m above the flume bottom. The test reach was composed of movable bed with a bed soil material to a depth of 0.05 m. A solid floor bed with a depth of 0.05 m locked up the bed soil material. The downstream section consists of a water level control sluice gate at the out flow point of the channel. To represent the erodible bed the rear portion of channel is filled with sand with  $D_{50} = 0.58\text{mm}$ . A point gauge operating in a flume measured the water depth and the scour hole dimension.

## DESIGN OF DIFFERENT TYPES OF STILLING BASIN

Twenty-six model of wooden are made to form the stilling basin as given in Fig (3). The design shapes of stilling basin have been based upon field experience works, which have not been incorporated in established design procedure. To simplify the investigations the length and the depth of all stilling basins are considered constant 25 cm and 1.50cm respectively.

## EXPERIMENTAL RESULTS

To study the performance of each type of stilling basins three discharges are considered ( $q = 51.26, 70.38$ , and  $86.96 \text{ cm}^3/\text{sec/cm}$ ). For each discharge, three water depths downstream the structure are used. The procedure followed in each test of this experiments was to first establish a flow. Also, the sandy soil was firmly packed and leveled horizontally before each experimental run. Seventy-eight runs were conducted, for each run, backwater feeding is started first until its depth reaches higher than required downstream water depth  $D$ , then upstream feeding is started. Three hours after many trials was chosen as a constant time for all runs. After this time, there was no appreciable change in scour hole dimensions. After the running time, the run was stopped and the flume was evacuated. The length and depth of scour hole profile along the centerline of the flume was recorded by a point gauge immediately downstream the basin. The results of the measurements and computations are tabulated in Table (1). The measured quantities are tabulated as follows discharge per centimeter width of flume, tail water depth, length of scour hole at the deepest point of scour and the whole length of scour hole. The dimensions parameters  $d/D$ ,  $L_s/D$ ,  $L_m/D$ , and  $1/F_r^2$  were computed.

## ANALYSIS

Experimental results were expressed in dimensionless forms and graphically represented to study the performance of different type of suggested stilling basin on scour hole dimensions. The relation between the dimensionless maximum scour depth ratio ( $d/D$ ) and the dimensionless ratio ( $1/F_r^2$ ) for different types of basin is shown in Fig. (4). The figures show that, ratio ( $d/D$ ) is inversely proportional to the ratio ( $1/F_r^2$ ). The respective figures also show that; for types of basin No. (5,7,15,16,17,18,19, 25,26) gives the smallest values of the ratio ( $d/D$ ), and type of basin No. (2,4,8,10,13,18,20,32) gives the greatest value of this ratio. The type of basin No. (1,2,23) gives the smallest values of ratio ( $d/D$ ) when ( $1/F_r^2$ ) more than 22.

Fig. (5) presents the relation between the scour length ratio ( $L_s/D$ ) and the dimensionless ratio ( $1/F_r^2$ ) for different types of basin. The figures show that ratio ( $L_s/D$ ) is also inversely proportional to the ratio ( $1/F_r^2$ ). It is noted that for type of basin No. (5,7,15,16,17,18,19,25,26) gives the smallest values of the scour length ratio ( $L_s/D$ ) while the greatest scour length ratio occurs when used type of basin No. (2,4,8,10,13,18,20,32). As shown in Fig. (5) the greatest values of scour length ratio occur when used basin No. (1,4,8) and ( $1/F_r^2$ ) less than about 6.

The distance from the end of the floor to the point of maximum scour depth,  $L_m$  is recorded and used to illustrate the variation of  $L_m/D$  with ( $1/F_r^2$ ). It has been observed that the dimensionless ratio ( $L_m/D$ ) and is inversely proportional to the ratio ( $1/F_r^2$ ). Fig. (6) shows that the type of basin No. (2,4,7, 8,16,17,26) gives the greatest values of ratio ( $L_m/D$ ) when the ratio ( $1/F_r^2$ ) is less than 6, and the type basin No. (4,10,13,21) have the smallest value of ratio  $L_m/D$ .

The results show that the scour length ratio ( $L_s/D$ ) is directly proportional to the scour depth ratio ( $d/D$ ) for all types of different basins. It is also noted that, the type of basin No. (1,2,3,4) gives the greatest scour length ratio when the scour depth ratio is about 0.8, while the smallest scour length ratio is evident when used type of basin No. (13,14,15,16) with scour depth ratio is about 0.2.

## CONCLUSIONS

The following may now be concluded from the results of the present research:

- 1- The effect of the variation of the shape of stilling basins having same length and depth on the scour hole dimensions is relatively small.
- 2- Increasing Froude number increases the maximum scour depth.
- 3- The distance from the end of the floor to the point of maximum scour depth is directly proportional to the scour length.
- 4- The variation of the shape of end sill of stilling basin is controlling the amount of energy dissipated within the vicinity of the stilling basin.
- 5- The effect of the variation of sloping bed of stilling basin on the scour hole dimension is relatively small when compared with the respective effect of the variation of the shape of end sill.

## REFERENCES

- Baghdadi, H. B.** (1997) "Local Sour Downstream Drop Structure." Alexandria Engineering Journal, Vol. 36, NO. 2, March 1997
- Bradley J. N. and Peterka A. J., October (1957)** "hydraulic design of stilling" A.S.C.E. Journal of The Hydraulics, Division Vol. 83.
- Breusers, H. N. C., and Raudkivi, A. J. (1991)** "Scouring, hydraulic structures manual. A A. Balkema, Rotterdam, The Netherlands
- Hoffmans, G. J. C. M. (1998)** "Jet scour in equilibrium phase." Journal of Hydraulic Engineering, ASCE, Vol. 124 No. HY 4, PP 430 – 437.
- Laursen, Emmett M., and Matthew, W. Flick,(1983)** "Scour at Sill Structures," Report No. FHWA/AZ 83/184, Arizona Transportation and Traffic Institute, College of Engineering the University of Arizona,
- Mohamed Ali, H. S.,(1985)** "Study of Efficiency of Stilling Basin D.S. of Low Head Irrigation Structures," thesis presented to Ain Shams University, at Cairo, Egypt, in partial fulfillment of the requirements for the degree of Master of Science,
- Peterka, A.J., (1978),** "hydraulic design of stilling basin and energy dissipators", U.S.dpt. Of the Interior, Bureau of Reclamation, Washington, D. C.
- Pillai N. N., and et al (1989).** "Hydraulic Jump Type Stilling Basin For Low Froude Numbers."Journal of Hydraulic Engineering, ASCE, Vol. 115 No. HY 7, PP 989-994.
- Uymaz, A. (1988)** "The investigation of the scours originating when water passes simultaneously over and vertical gates." Journal of Hydraulic Engineering, ASCE, Vol. 114 No. HY 7, PP 811 – 816.
- U. S. Bureau of Reclamation, (1977)** Design of Small Dams, A Water Resources Technical Publication, Denver Colorado, Second Edition,.

Table (1) Experimental data for twenty-six type of stilling basin

Type	$q = 51.26 \text{ cm}^2/\text{cm/sec}$			$q = 70.38 \text{ cm}^2/\text{cm/sec}$			$q = 86.96 \text{ cm}^2/\text{cm/sec}$			
	D	d <sub>1</sub>	d <sub>2</sub>	L <sub>m</sub> /D	L <sub>m</sub> /D	L <sub>m</sub> /D	L <sub>m</sub> /D	L <sub>m</sub> /D	L <sub>m</sub> /D	
1	2.1	0.7	0.171	3.5	2.439	3.73	4.6	7	0.217	10.5
1	2.9	1.1	0.379	7	2.414	1.0	2.821	19	4.13	2.293
1	2.1	1.5	0.714	7.8	3.714	1.4	6.667	2.9	2.1	0.385
2	2.9	1	0.345	8	2.759	11.8	4.069	9.1	1.7	0.426
2	2.1	1.4	0.667	7	3.333	1.3	8.19	3.556	2.2	0.759
4	4.1	0.9	0.22	6	1.463	9	2.195	25.73	4.6	1.3
2	2.9	1.1	0.379	6	2.089	10	3.448	9.1	1.7	0.426
3	3	2.1	1.3	0.619	6.5	3.695	12	5.714	3.356	2.9
4	4.1	0.85	0.207	6.5	1.588	10.9	2.659	25.73	4.6	1.35
4	2.9	1	0.345	7	2.414	12	4.138	9.1	1.8	0.462
4	2.1	1.4	0.667	7.9	3.762	1.4	6.667	3.556	2.9	2.5
4	4.1	0.9	0.22	7	1.707	1.1	2.663	25.73	4.6	1.4
5	2.9	1.2	0.414	7	2.414	12	4.139	9.1	1.7	0.436
5	2.1	1.3	0.619	7.5	3.571	1.3	6.19	3.356	2.9	2.3
5	4.1	0.8	0.195	6	1.463	10	2.439	25.73	4.6	1.4
6	2.9	1	0.345	6.9	2.379	1.1	3.793	9.1	1.6	0.41
6	2.1	1.35	0.643	7.8	3.714	13.2	6.286	3.356	2.9	2.15
6	4.1	0.9	0.22	7	1.707	10	2.439	25.73	4.6	1.3
7	2.9	1.1	0.379	7	2.414	12	4.138	9.1	1.7	0.436
7	2.1	1.4	0.667	8	3.81	14	5.867	3.356	2.9	2.25
8	2.9	1.2	0.414	7.3	1.707	11	2.663	25.73	4.6	1.2
8	2.1	1.3	0.614	8	3.81	14.5	6.505	3.356	2.9	2.2
8	4.1	0.8	0.195	6	1.483	10	2.439	25.73	4.6	0.9
9	2.9	1.1	0.379	6.5	2.241	10.8	3.724	9.1	1.6	0.395
9	2.1	1.6	0.762	7.2	3.429	12.5	5.852	3.356	2.9	2.0
9	4.1	0.9	0.22	6.5	1.585	10.1	2.483	25.73	4.6	1.1
10	2.9	1.2	0.414	7	2.414	11	3.733	9.1	1.5	0.385
10	2.1	1.35	0.646	7	2.414	10.2	3.517	9.1	1.4	0.359
10	4.1	1	0.244	6	1.463	10.5	2.561	25.73	4.6	1.25
11	2.9	1	0.345	6.5	2.241	11	3.733	9.1	1.6	0.41
11	2.1	1.6	0.762	7.5	3.571	12.8	6.095	3.356	2.9	2.35
11	4.1	1	0.244	7	1.707	10	2.439	25.73	4.6	1.0
12	2.9	1.2	0.414	7	2.414	11	3.733	9.1	1.4	0.374
12	2.1	1.4	0.667	8	3.81	13.5	6.429	3.356	2.9	2.1
12	4.1	0.9	0.22	8.8	3.81	13.5	6.429	3.356	2.9	2.1
13	2.9	1.35	0.646	7	2.414	11.5	3.571	13.2	6.286	3.356
13	2.1	1.5	0.714	7.5	3.571	13.2	6.286	3.356	2.9	2.22

Table (1) Continued

Type	q = 51.26 cm <sup>2</sup> /cm <sup>sec</sup>				q = 70.38 cm <sup>2</sup> /cm <sup>sec</sup>				q = 86.56 cm <sup>2</sup> /cm <sup>sec</sup>					
	D	d <sub>1</sub>	d <sub>2</sub> /D	L <sub>m</sub>	D	d <sub>1</sub>	d <sub>2</sub> /D	L <sub>m</sub>	D	d <sub>1</sub>	d <sub>2</sub> /D	L <sub>m</sub>		
4.1	0.8	0.985	6	1.463	10	2.439	25.73	4.6	1.2	0.261	10	2.174	17.3	
14	2.9	1	0.345	6.5	2.241	10.5	3.621	9.1	3.9	1.6	0.41	10.5	2.692	19
14	2.1	1.4	0.887	7	3.333	12	5.714	3.358	2.9	2.5	1.862	12	4.138	21.5
4.1	0.9	0.22	6.5	1.585	10	2.439	25.73	4.6	1.2	0.261	11.5	2.5	1.414	4.83
15	2.9	1.1	0.379	7	2.414	11	3.793	9.1	3.9	1.5	0.385	12	3.077	19.2
15	2.1	1.45	0.69	7.9	3.762	13.5	6.429	3.356	2.9	2.4	0.828	13	4.483	20
4.1	0.8	0.195	6	1.463	10	2.439	25.73	4.6	1.3	0.283	11.6	2.522	20.2	
16	2.9	1	0.345	6.5	2.241	11	3.793	9.1	3.9	1.5	0.41	12.1	3.103	21.9
16	2.1	1.35	0.843	7.2	3.429	12.8	6.095	3.356	2.9	2.7	0.931	12.9	4.448	23.8
11	0.8	0.195	5	1.22	9.8	2.39	25.73	4.6	1.3	0.283	10	2.114	18.2	
17	2.9	1.1	0.379	5	2.124	11	3.793	9.1	3.9	1.7	0.436	11	4.821	21.5
17	2.1	1.3	0.619	5.75	2.738	13	6.19	3.356	2.9	2.6	0.839	12	4.139	23.5
4.1	1	0.244	7.2	1.756	10.5	2.561	25.73	4.6	1.4	0.304	13	2.826	20	
18	2.9	1.3	0.448	7	2.414	12	4.138	9.1	3.9	1.8	0.462	13	3.333	22
18	2.1	1.6	0.762	8	3.81	14	6.667	3.356	2.9	2.5	0.862	14	4.838	25
4.1	1.1	0.268	6	1.463	9.8	2.39	25.73	4.6	1.5	0.326	10.5	2.283	19	
2.9	1.2	0.414	7	2.414	10.5	3.621	9.1	3.9	1.7	0.436	11	2.827	20	
19	2.1	1.5	0.714	7.5	3.571	11.8	5.619	3.356	2.9	2.4	0.828	12	4.138	22
4.1	1.15	0.38	7	1.707	10.5	2.561	25.73	4.6	1.4	0.304	13	2.826	18.2	
20	2.9	1.3	0.448	7	2.414	11	3.793	9.1	3.9	1.7	0.436	13.2	3.385	19.5
20	2.1	1.58	0.752	7.9	3.762	14	6.667	3.356	2.9	2.3	0.793	14	4.828	24.5
4.1	1	0.244	7	1.707	10.5	2.439	25.73	4.6	1.6	0.348	11	2.351	18.1	
2.9	1.3	0.448	7.5	2.586	10.5	3.621	9.1	3.9	1.7	0.436	11	2.821	20	
21	2.1	1.6	0.762	8	3.81	12	5.714	3.356	2.9	2.4	0.828	11	2.462	17.5
4.1	0.8	0.195	6.9	1.683	10.1	2.463	25.73	4.6	1.6	0.348	11	2.391	18.5	
22	2.9	1.1	0.379	7.1	2.448	11.5	3.966	9.1	3.9	1.75	0.449	12.0		
22	2.1	1.4	0.687	8	3.81	13.1	6.238	3.356	2.9	2.5	0.862	12.2	4.267	23.6
4.1	0.6	0.146	8	1.951	10.8	2.634	25.73	4.6	1.4	0.304	11	2.391	19.3	
2.9	1.1	0.379	8.5	2.931	12.3	4.241	9.1	3.9	1.8	0.452	11	2.821	20.1	
23	2.1	1.5	0.714	9.1	4.333	15.5	7.381	3.356	2.9	2.2	0.759	12.5		
4.1	0.8	0.195	6	1.463	8	1.951	25.73	4.6	1.7	0.377	12.8			
2.9	1.1	0.379	6.5	2.241	11	3.793	9.1	3.9	1.9	0.487	13.1			
24	2.1	1.45	0.69	7.8	3.714	13.1	6.238	3.356	2.9	2.3	0.793	14		
4.1	0.8	0.195	7.5	1.829	13.5	3.293	25.73	4.6	1.3	0.283	11.5			
2.9	1.1	0.379	8	2.759	14	4.828	9.1	3.9	1.7	0.436	12.5			
25	2.1	1.3	0.619	9.2	4.381	15.1	7.19	3.356	2.9	2.2	0.759	13		
4.1	1	0.244	9	2.185	11	2.633	25.73	4.6	1.1	0.239	12.5			
2.9	1.1	0.379	10	3.448	12	4.138	9.1	3.9	1.8	0.482	13			
26	2.1	1.4	0.667	10.5	5	13.5	6.429	3.356	2.9	2.4	0.828	13.2		

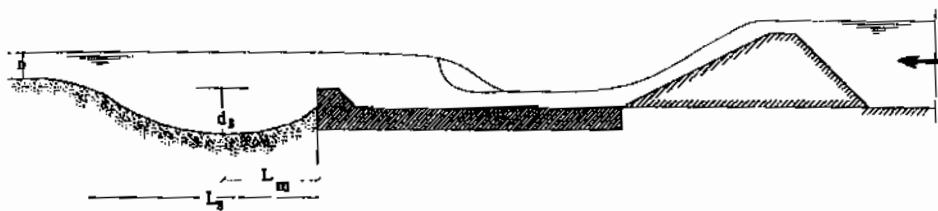
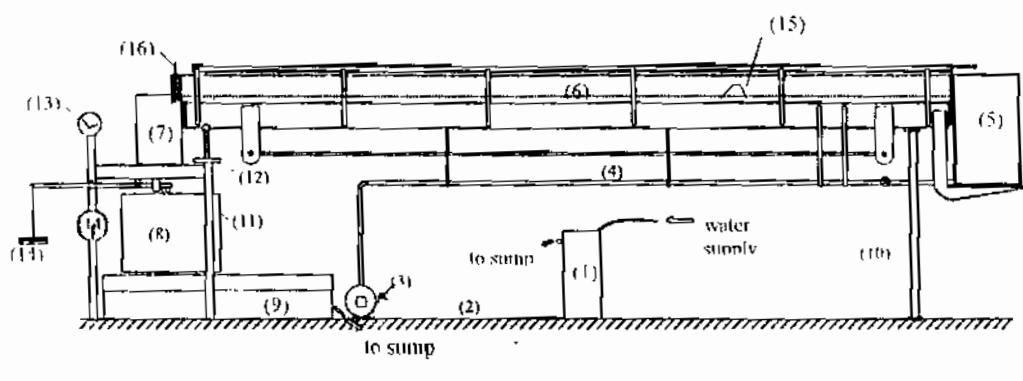


Fig. (1) : Definition sketch for basin test



- |                              |                     |                       |                     |
|------------------------------|---------------------|-----------------------|---------------------|
| 1) Constant head tank (1)    | 6) Movable bed sand | 11) D.S. carrier      | 16) Tail gate       |
| 2) Supply pipe from tank (1) | 7) Tail tank        | 12) Controlling screw | 17) Perfomed Screen |
| 3) 1 HP pump                 | 8) Weighing tank    | 13) Clock             |                     |
| 4) Supply pipe from pump     | 9) Sump             | 14) Weights           |                     |
| 5) Constant head tank (2)    | 10) U.S. carrier    | 15) Weir              |                     |

Fig. (2) General arrangement of the experimental set-up

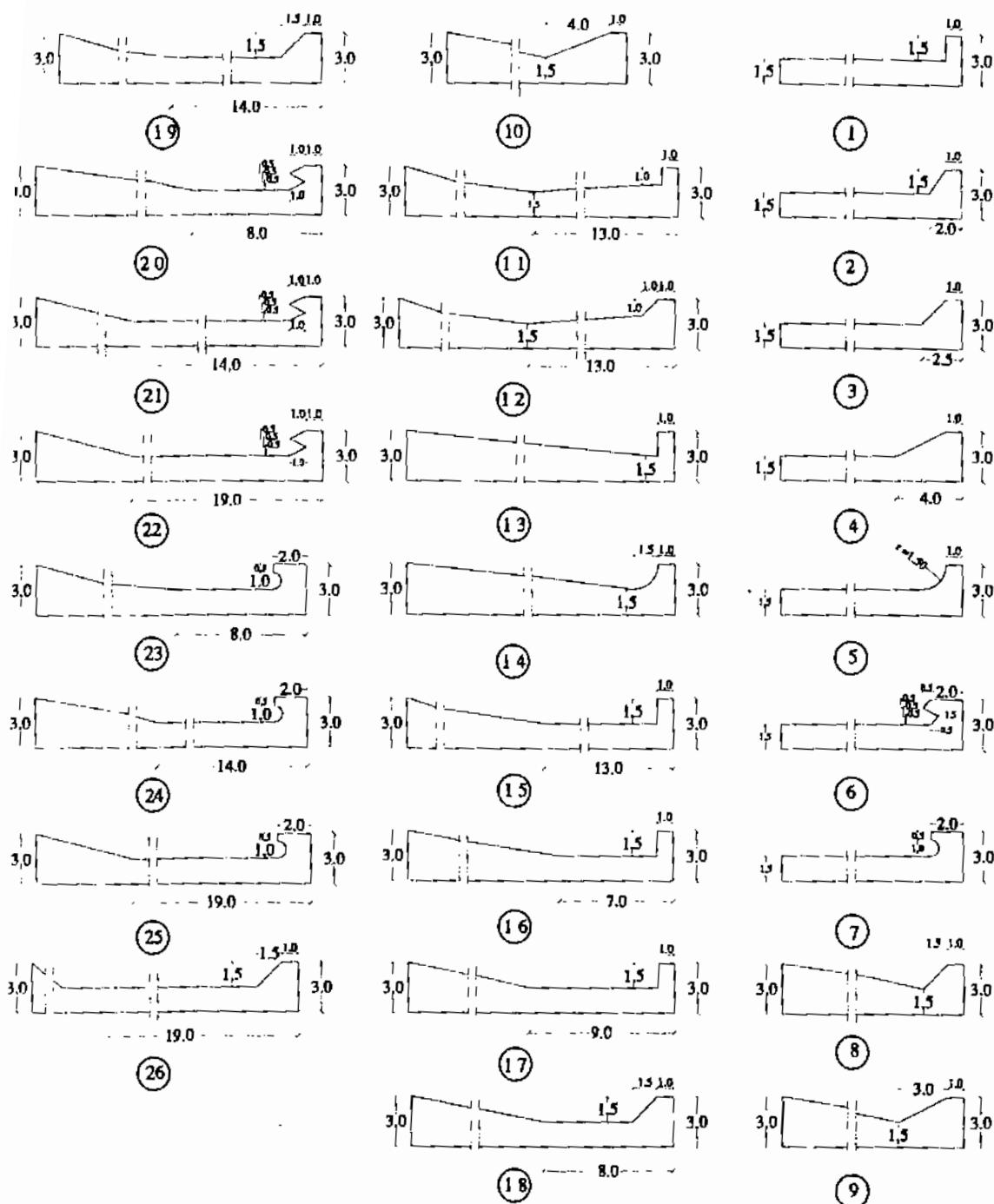
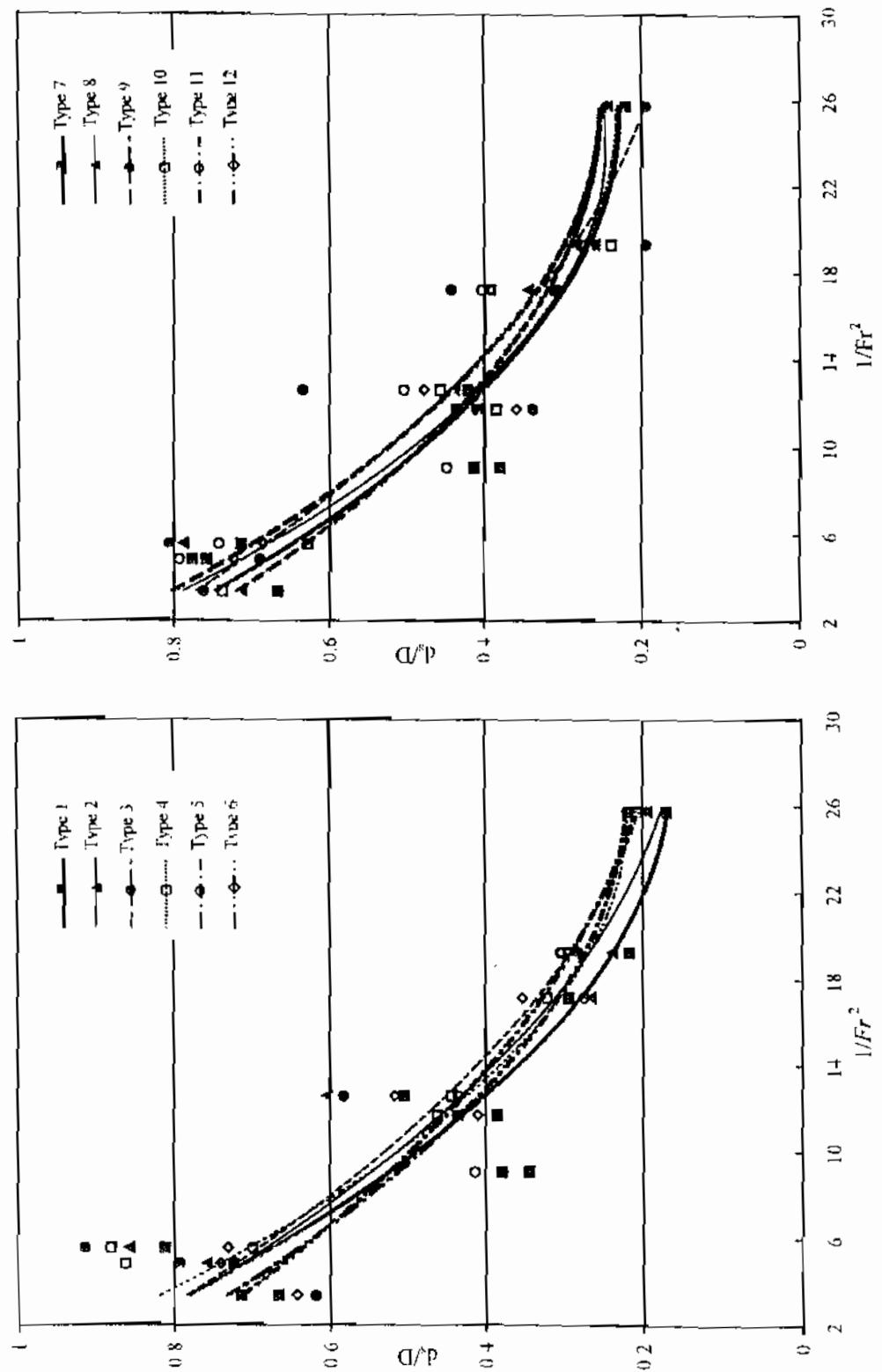


Fig (3) : Different types of stilling basin used in the investigation  
( Dimensions in cm )

Fig. (4. a ) Variation of  $d_s/D$  with  $1/Fr^2$  for different types of snilling basins.

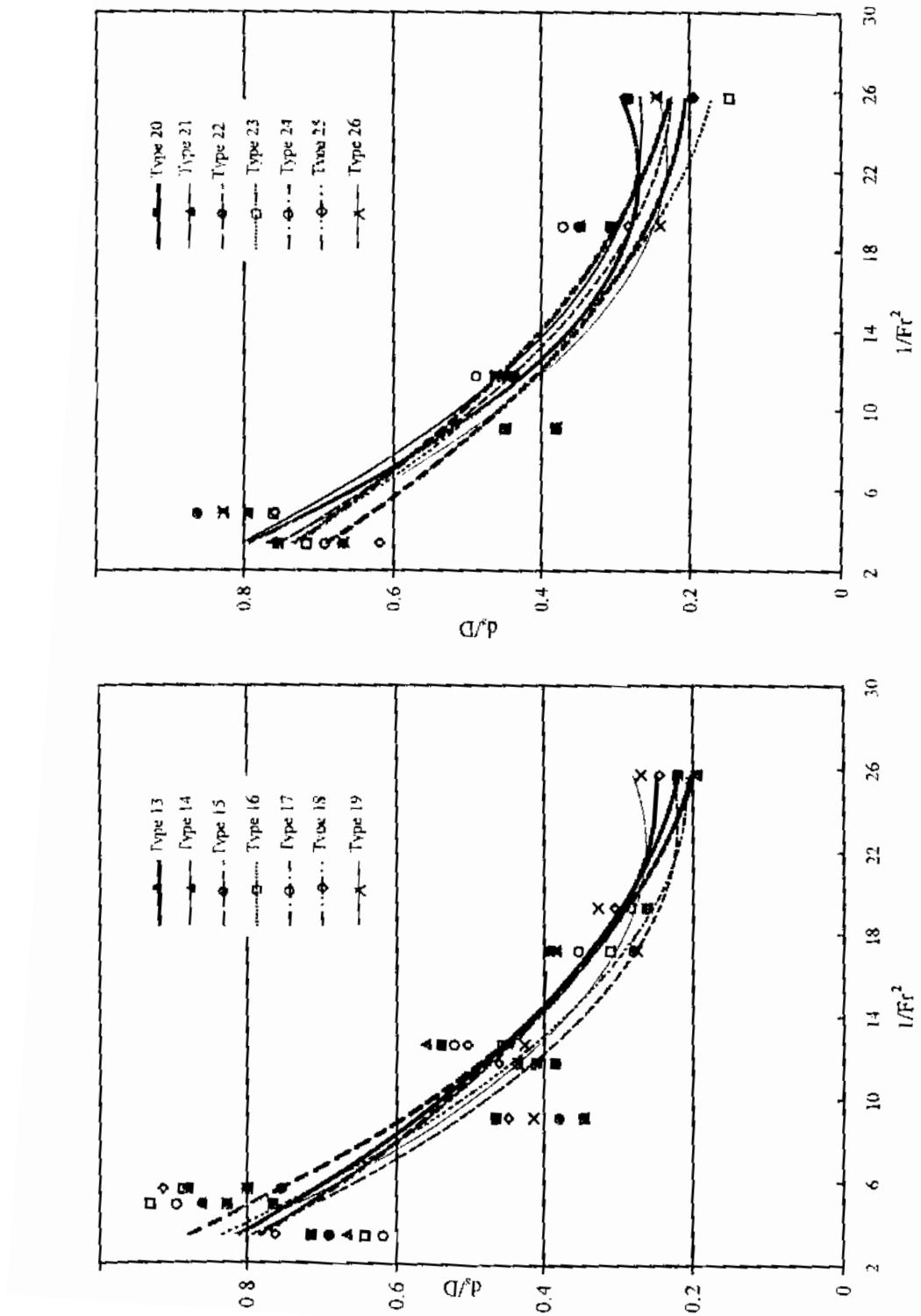
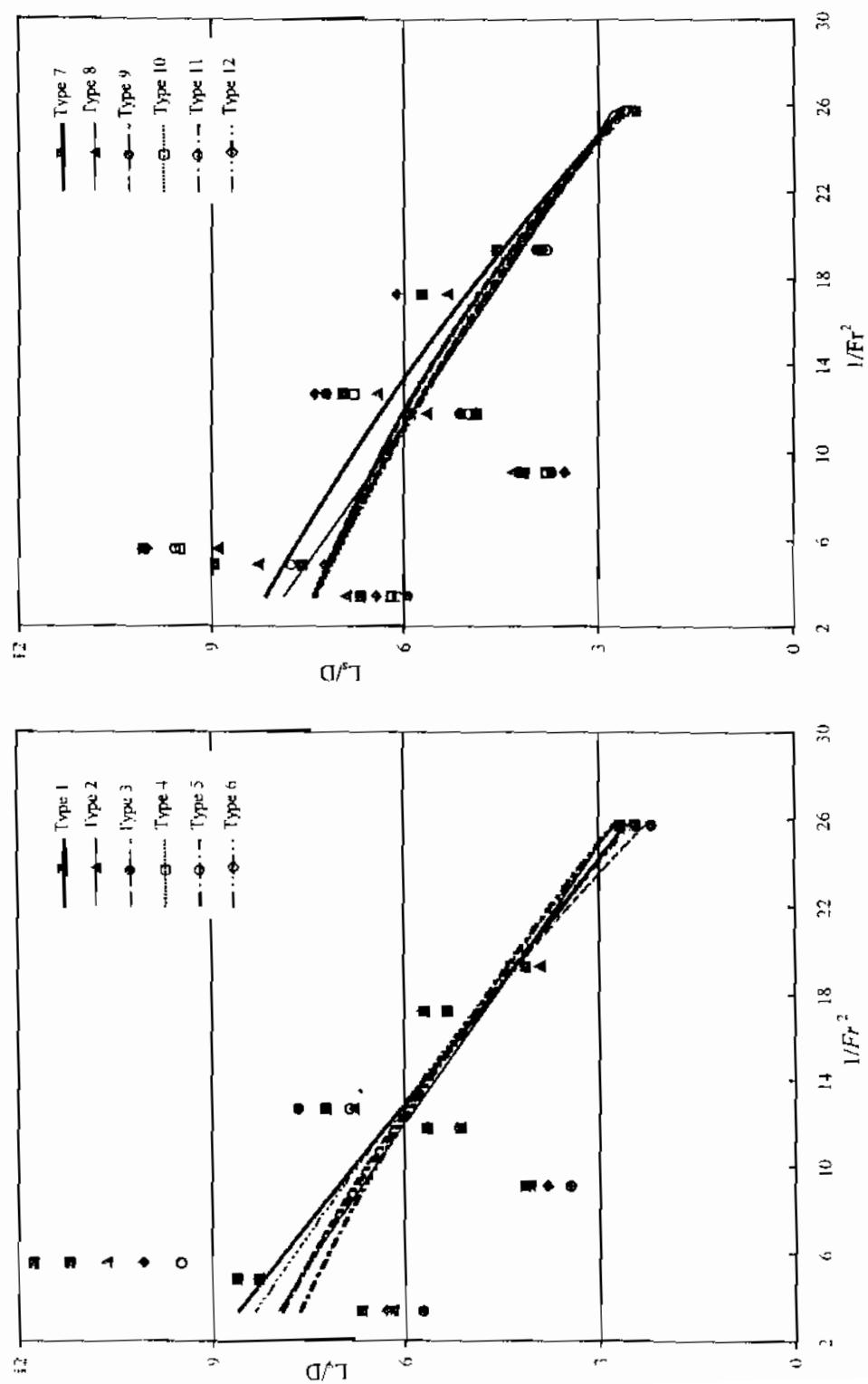


Fig. (4, b ) Variation of  $d/D$  with  $1/Fr^2$  for different types of stilling basins.

Fig. (5. a) Variation of  $L/D$  with  $1/Fr^2$  for different types of stilling basins.

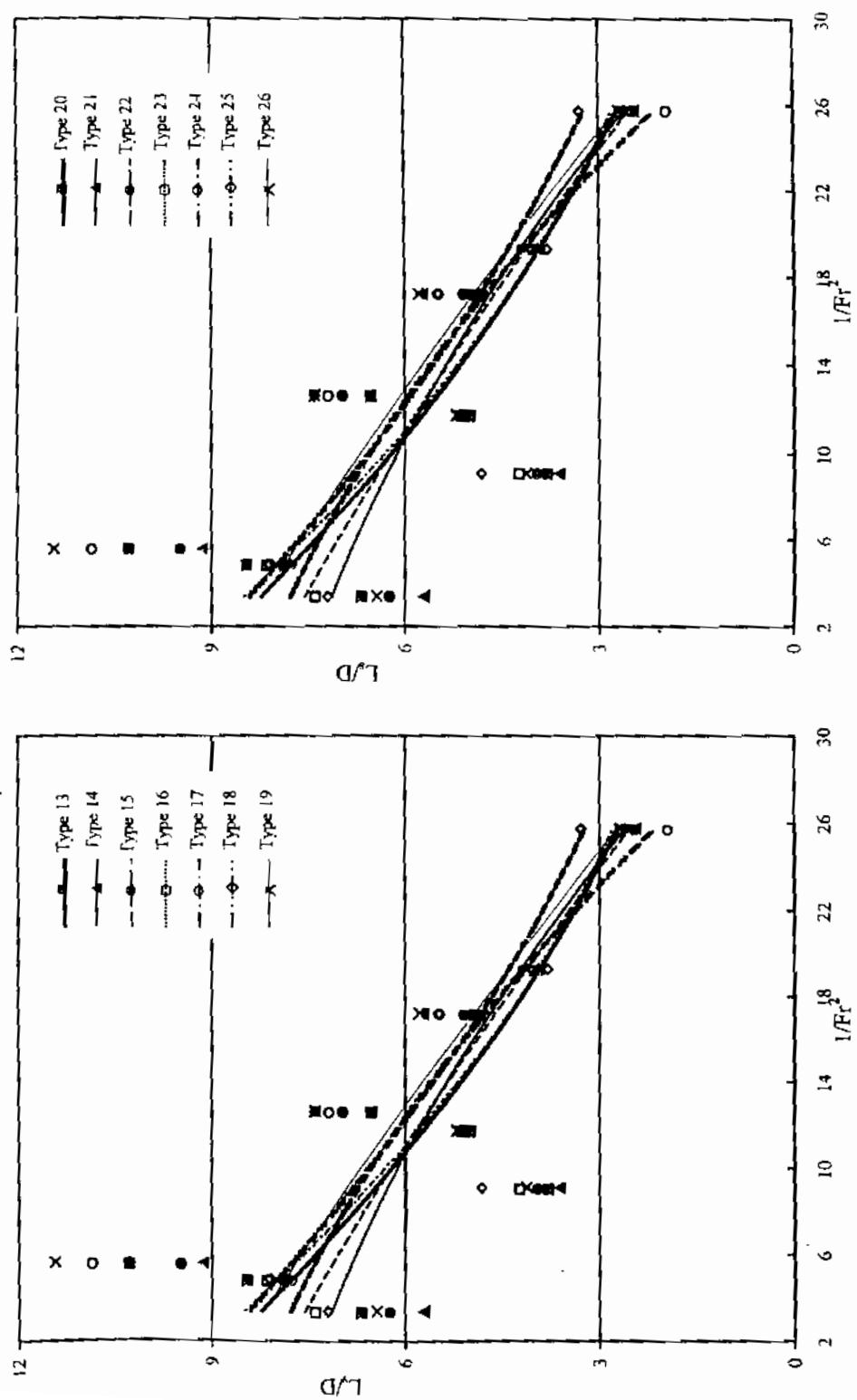


Fig. (5. b) Variation of  $L/D$  with  $1/Fr^2$  for different types of stilling basins.

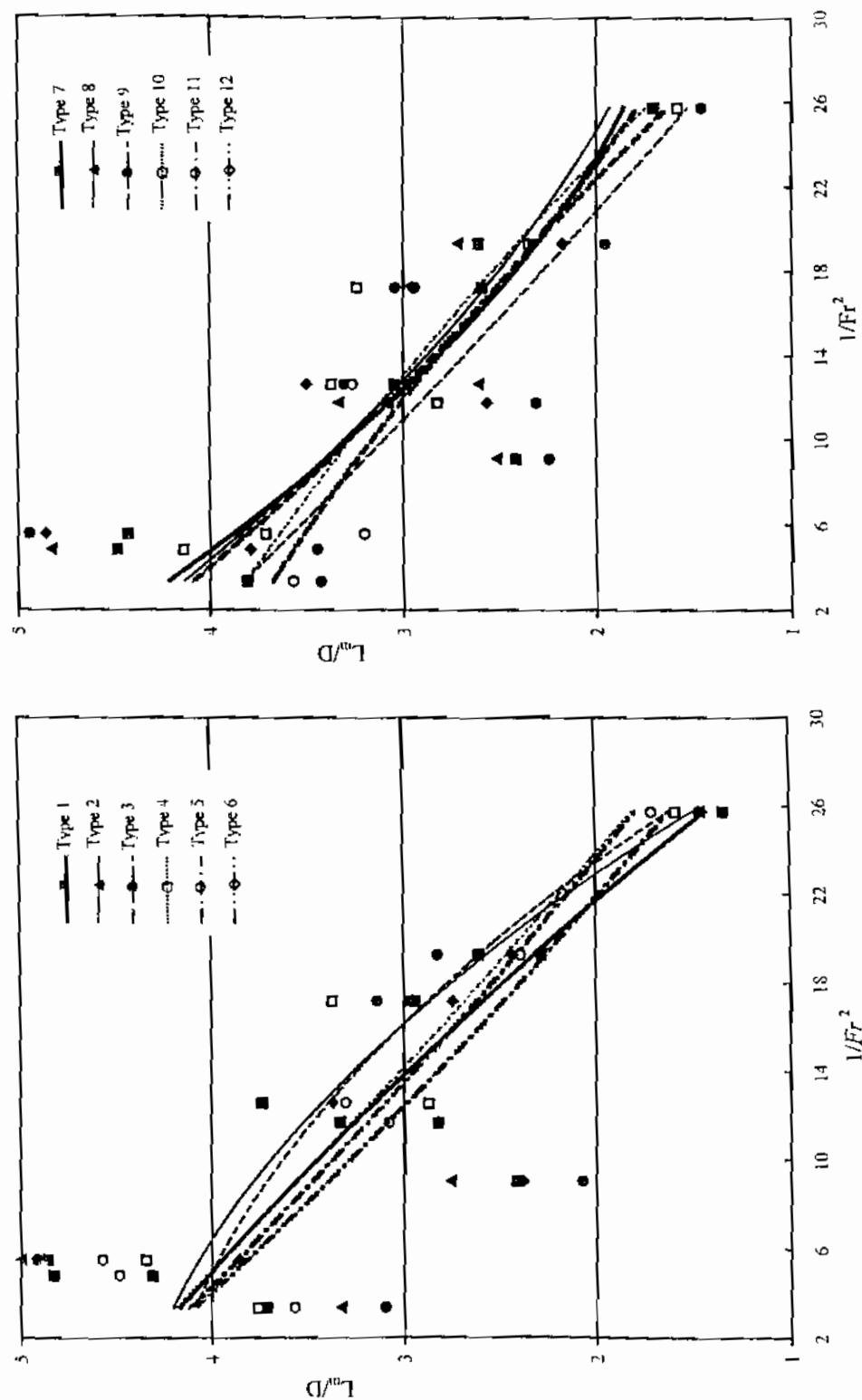


Fig. (6. a) Variation of  $L_m/D$  with  $1/Fr^2$  for different types of silting basins.

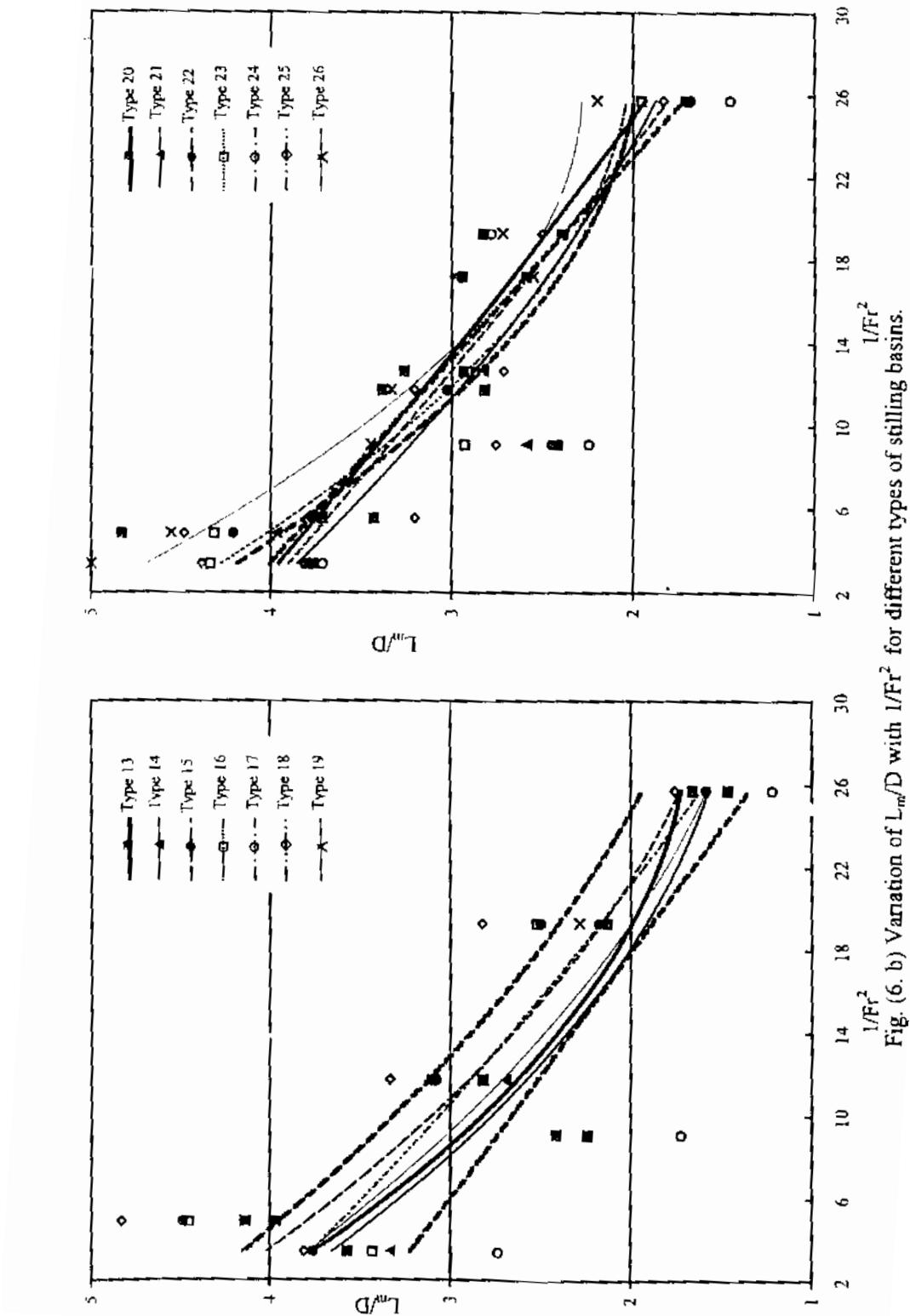


Fig. (6. b) Variation of  $L_m/D$  with  $1/Fr^2$  for different types of stilling basins.

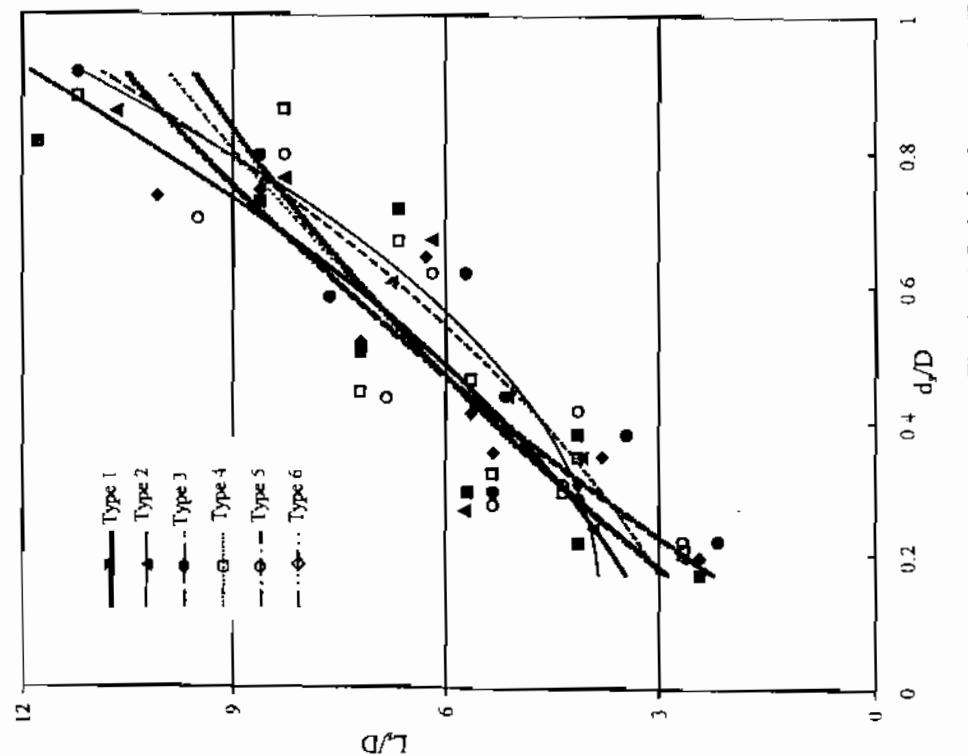
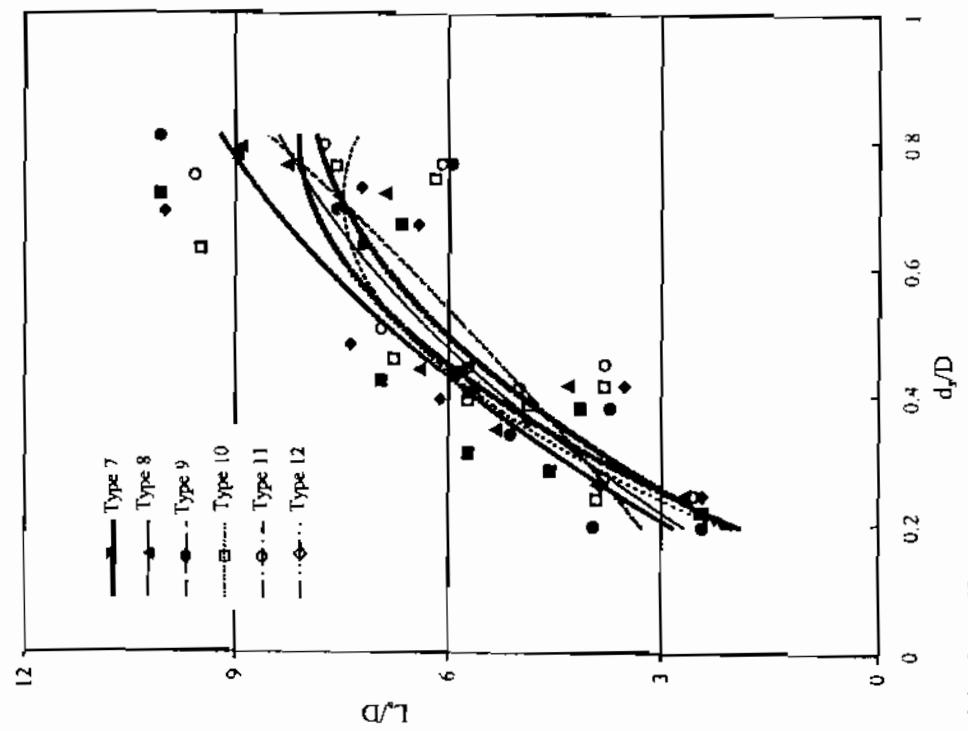


Fig. (7. a) Relation between  $L_f/D$  and  $d_f/D$  for different types of stilling basins.

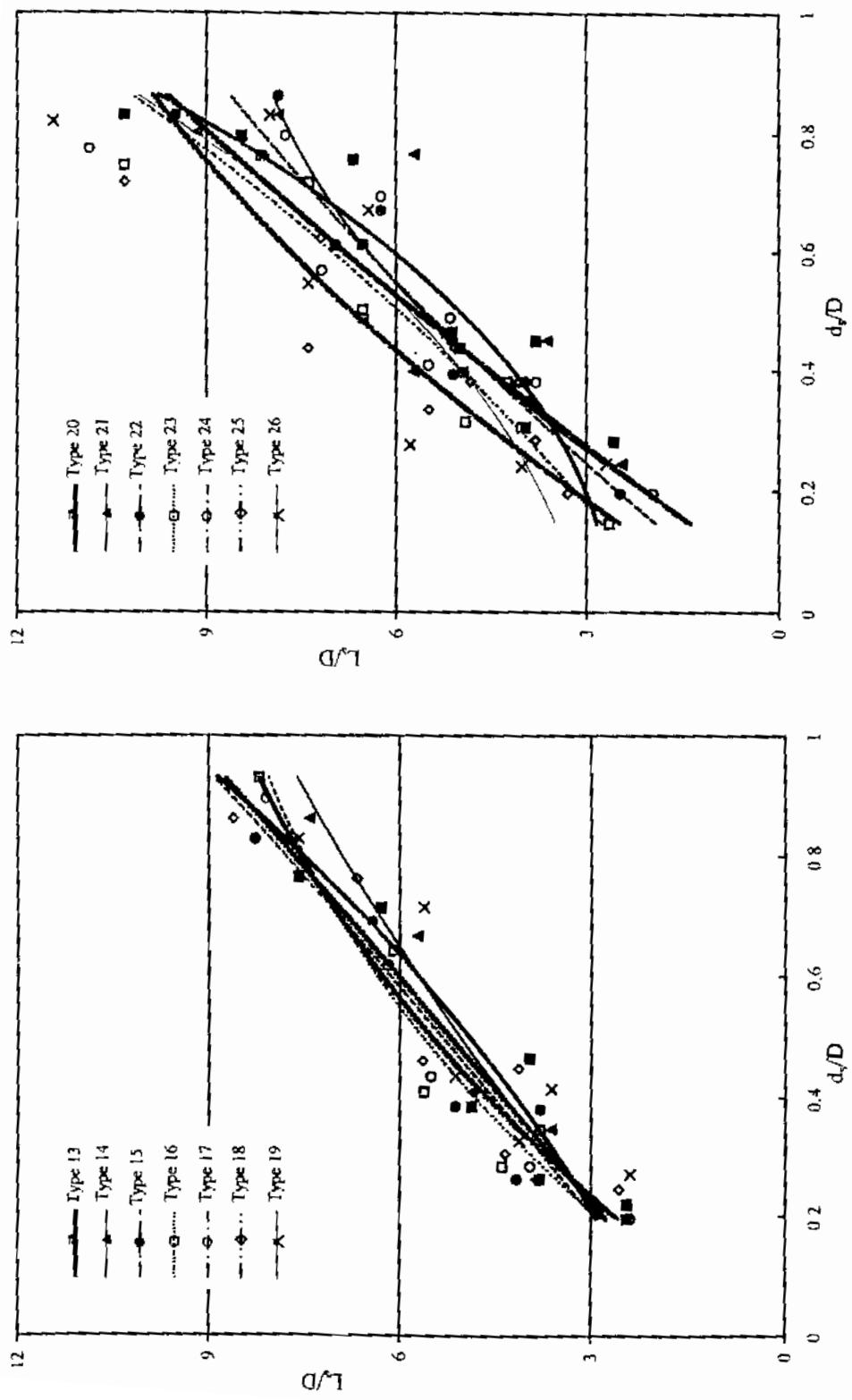


Fig. (7. b) Relation between  $L_s/D$  and  $d_s/D$  for different types of stilling basins.